

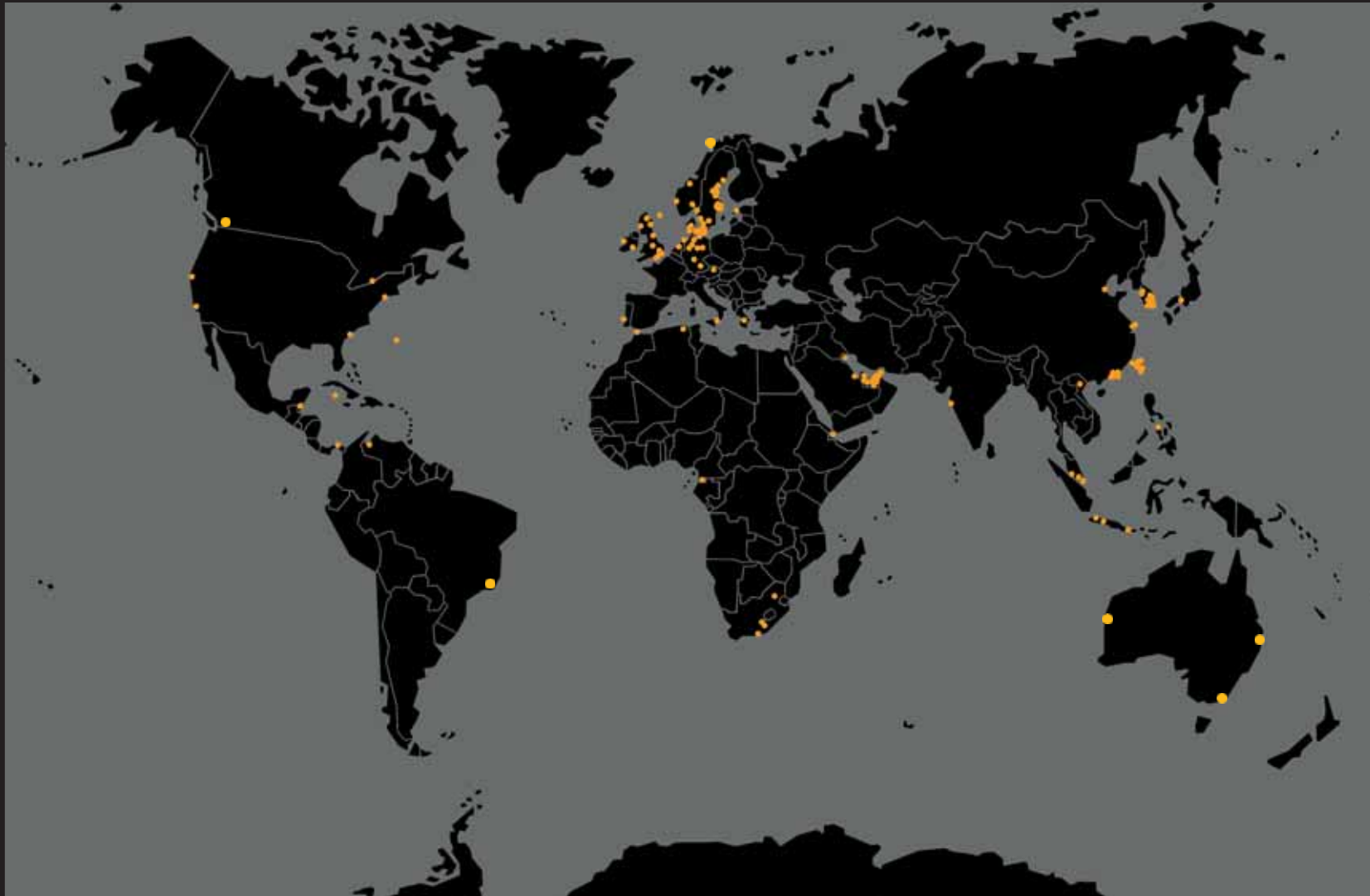
STORSTRØM BRIDGE

Jesper B. Henriksen

Partner at Dissing+Weitling - Mobility



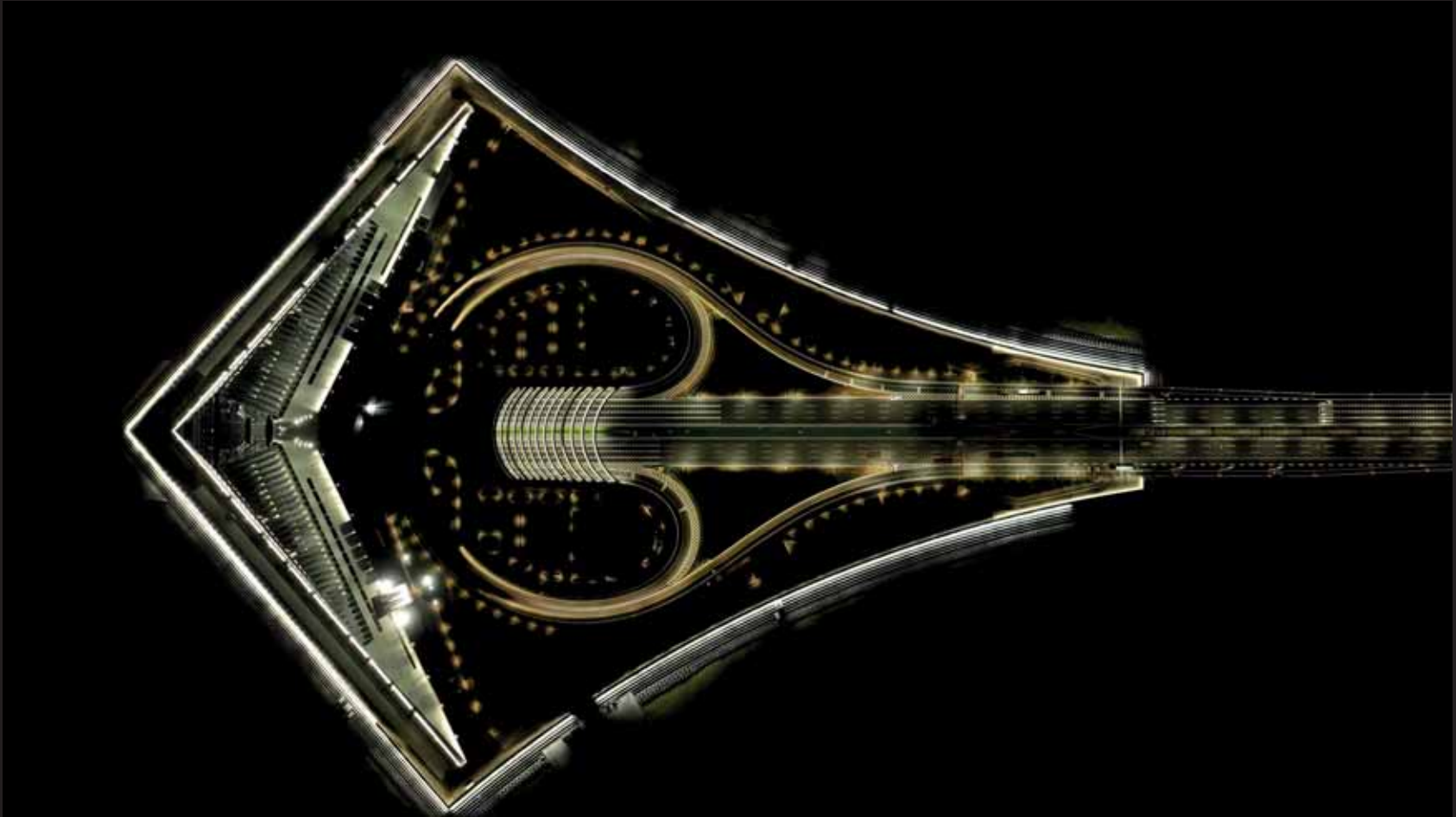
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Client:
The Danish Road Directorate

Advisory team:
COWI - DISSING+WEITLING - Hasløv & Kjærsgård

Contractor: SBJV (Itinera)



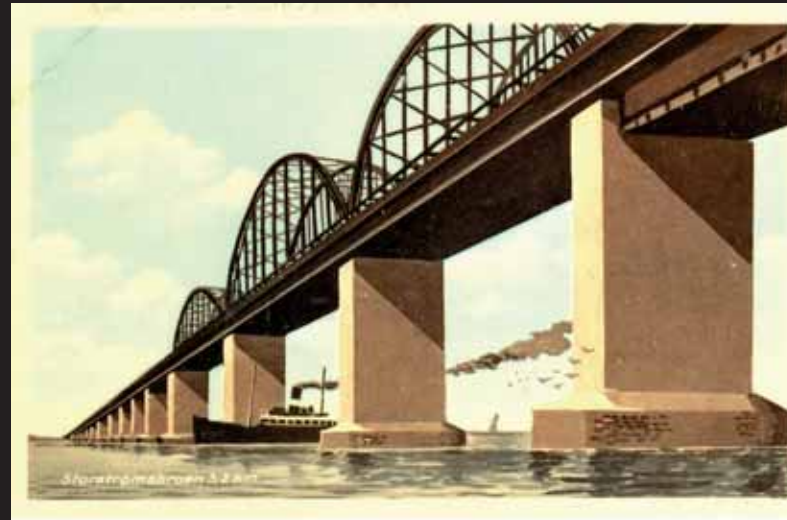
Replacement Project



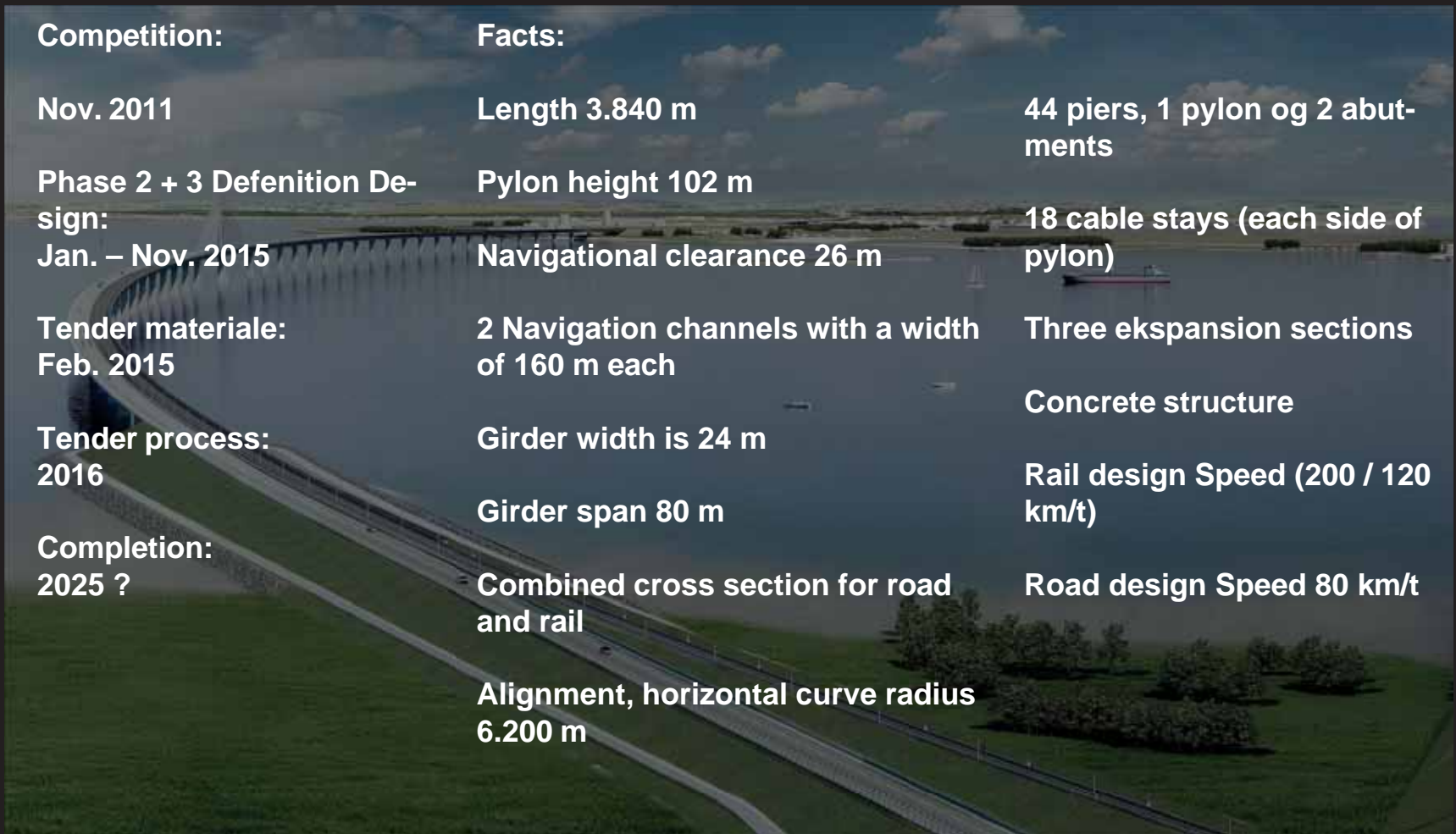
From 1937,
longest bridge
in Europe in
1938



A landmark in
Denmark



Bought in the UK, due to
too much Danish export
of butter and
Bacon



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SKETCHING OF DIFFERENT CONCEPTS 2011





OPTIMIZATION OF SECTION, SIMPLE CONSTRUCTION



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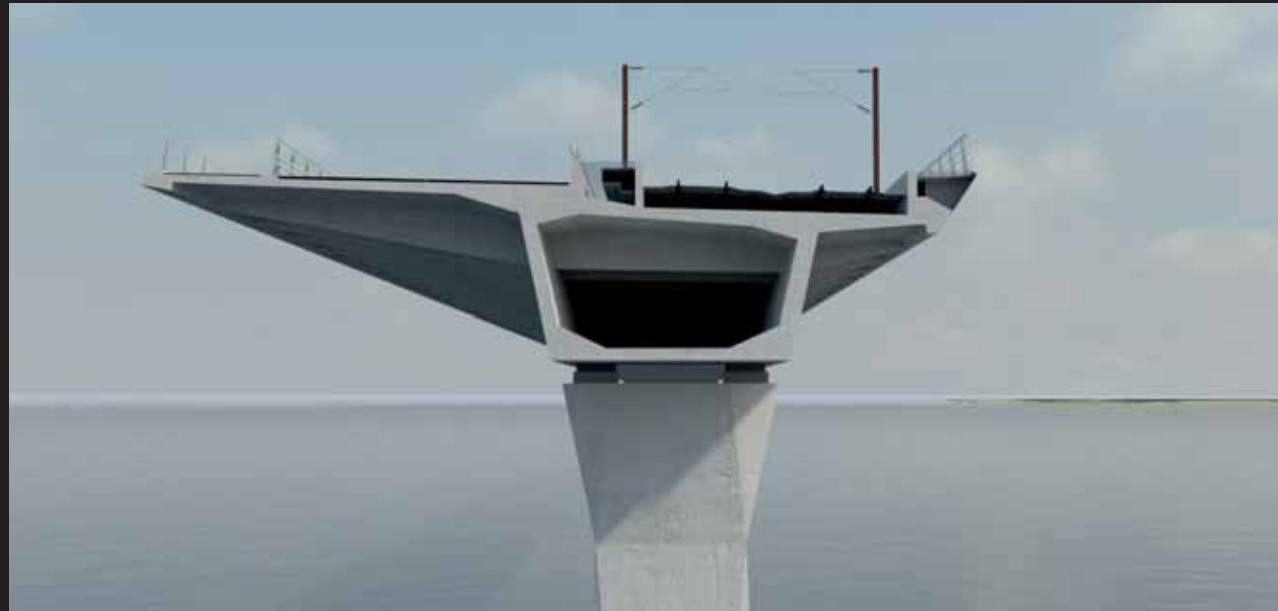
**“ASYMMETRICAL
CROSS SECTION”**

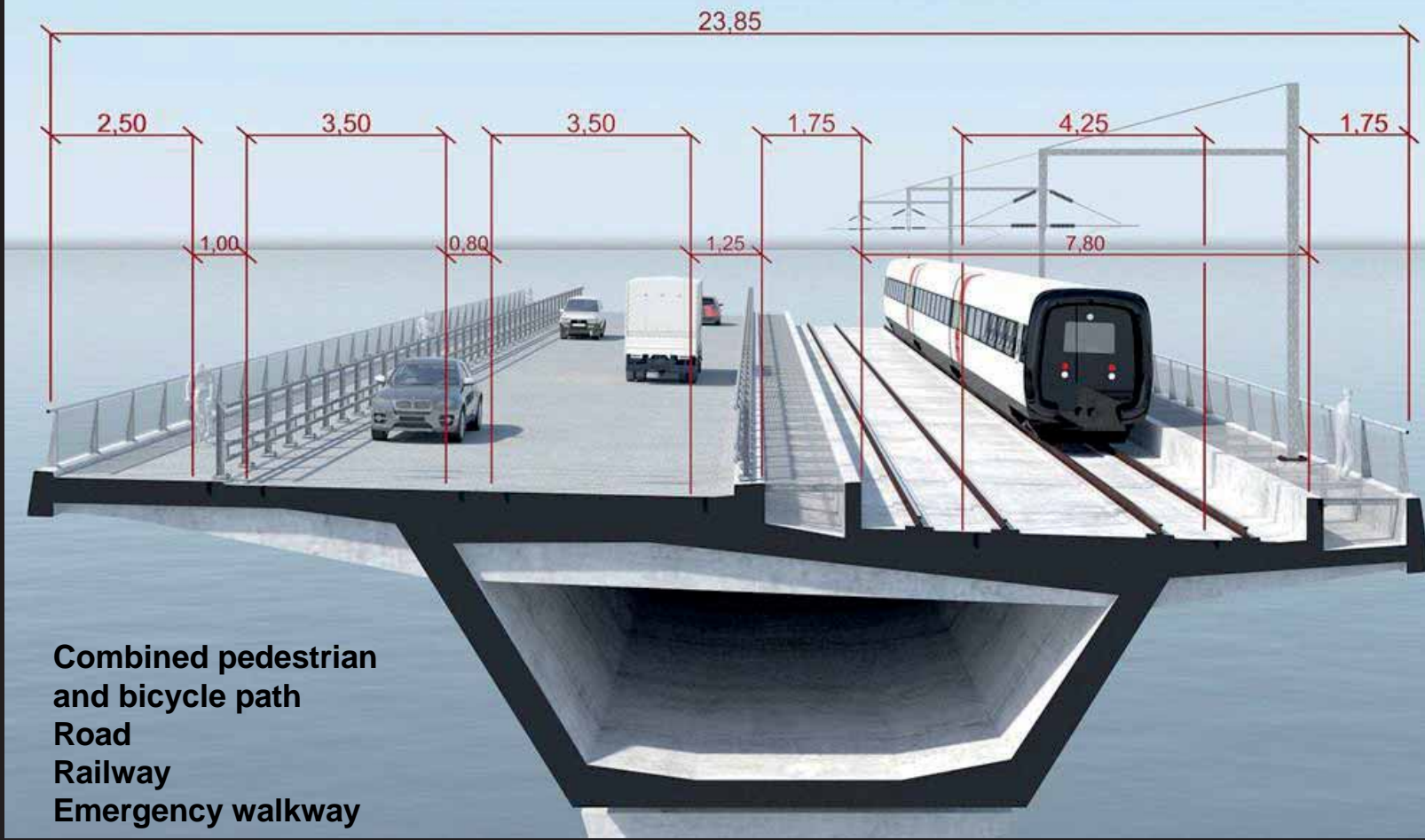
**GOOD USER EXPERI-
ENCE**

**LESS PIERS AND
FOUNDATIONS**

**OPTIMIZATION OF
MATERIALS**

**ELEGANT
EXPRESSION -
ARCHITECTURE**



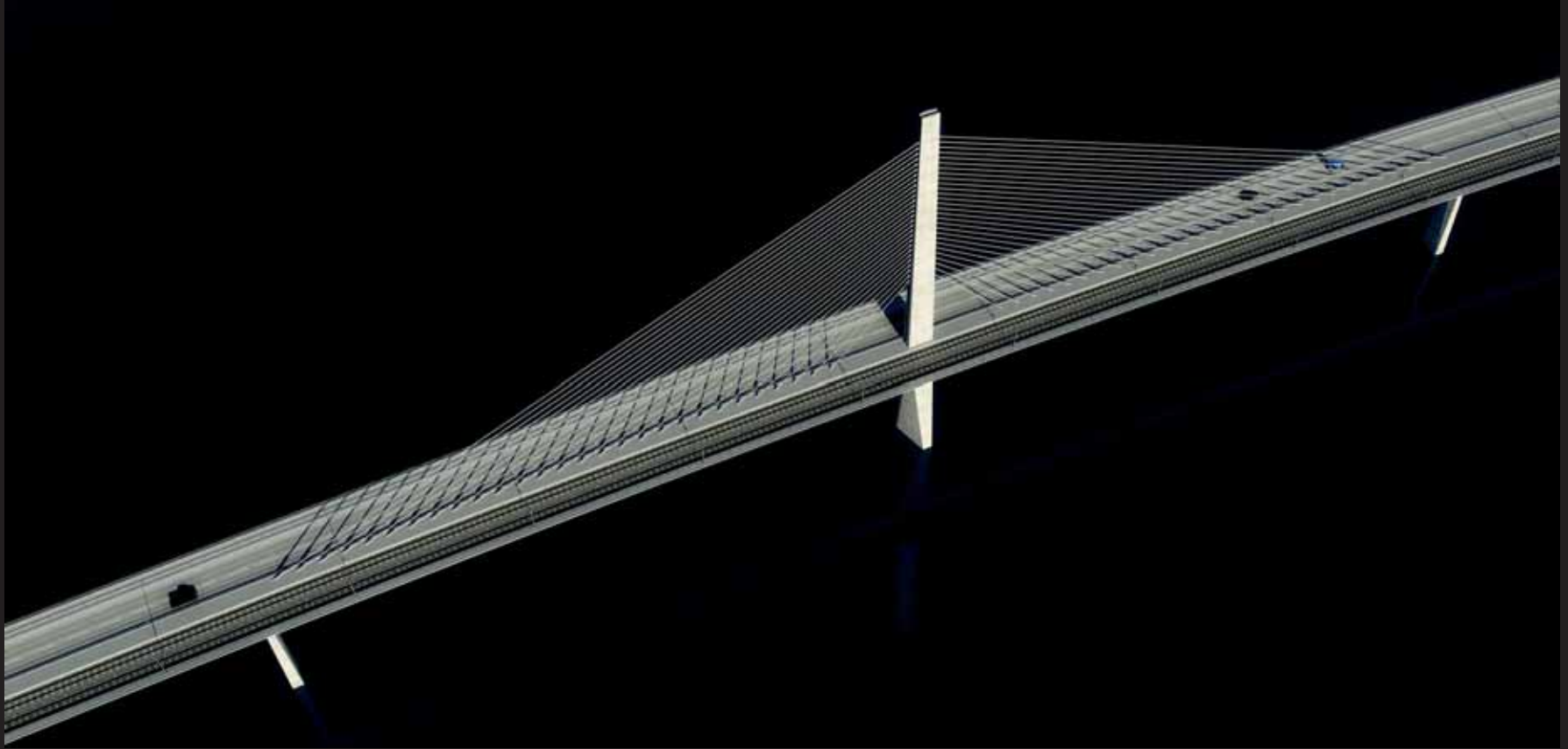




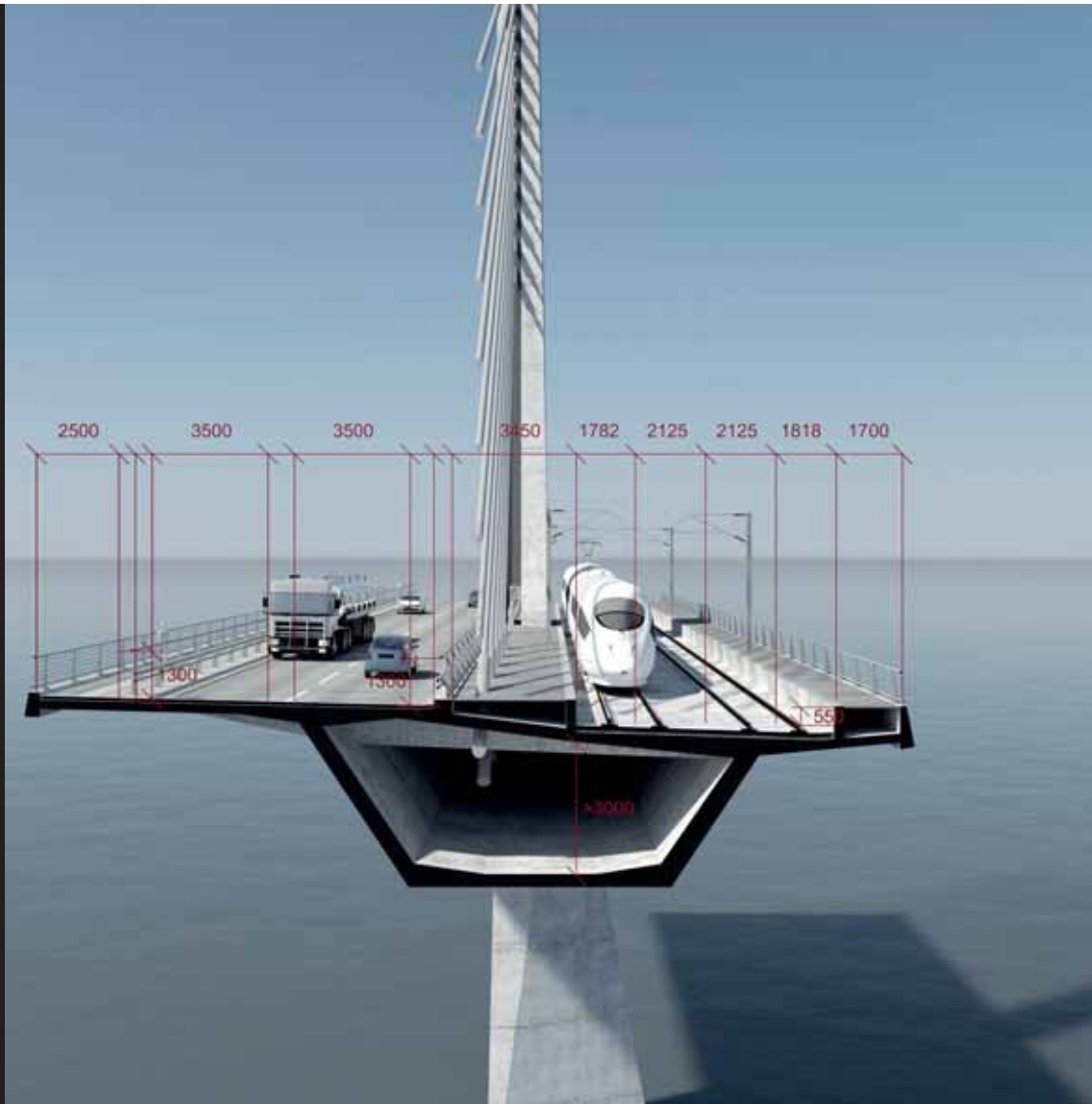
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2500 3500 3500 3450 1782 2125 2125 1818 1700

1300

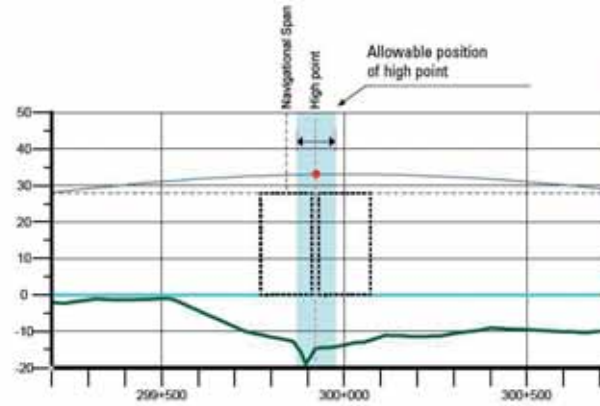
1300

155

3000

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Definition design



NAVIGATIONAL SPAN AND HIGH POINT PLACEMENT

2.2 NAVIGATION SPANS

The navigation spans shall be centred around the high point of the vertical alignment.

The navigational spans shall be min. 2x160m.
DBS01-1-2.2-310 MR

2.3 PROFILE

The minimum vertical radius over the navigation spans shall be min. 40000 m. (60000 m PMR)
DBS01-2.2-128 MR

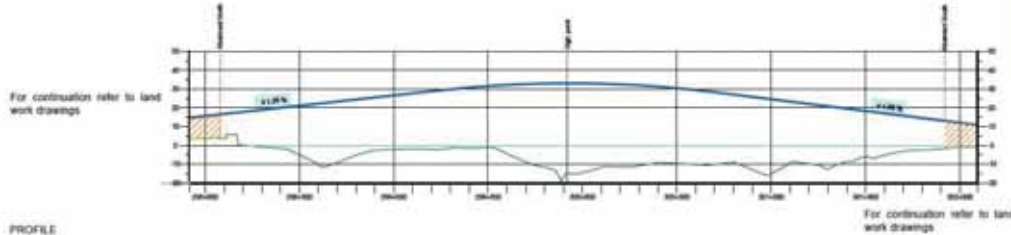
The maximum vertical radius over the navigation spans is 72000 m.
DBS01-0-2.2-125 R

The longitudinal profile for the reference line (PRL) shall be determined as follows:

The level to the lowest point of the girder soffit shall be ≥ 26.85 m (DVR90) within the navigation spans in load combinations SLS1 and SLS2.
DBS01-0-2.2-100 MR

Allowance for future sea level rise shall not be added.

The maximum allowable gradient on the railway on the viaducts is 1.25%.
DBS01-0-2.2-116 MR



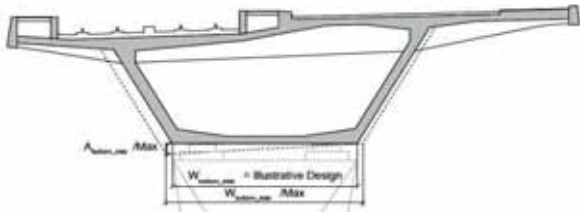
PROFILE

NAME OF PARAMETER	DESCRIPTION	STATIONING IN ILLUSTRATIVE DESIGN	DBS	ALLOWED VARIATIONS
ST _{High Point}	Station of high point	299+920	DBS01-0-2.2-070 MR	298+970 - 299+970

PROFILE/ DRAWINGS NOT TO SCALE

Design Parameters 2.2.A / Parameters of Design

NOTES



VALUE OF W_{bottom_slab}
 • Illustrative Design / 3000 mm
 • Max / 3500 mm
 DBS01-0-2.2-410 PMR

VALUE OF A_{bottom_slab}
 • Illustrative Design / Min / 0 %
 • Max / 3.33 %
 DBS01-0-2.2-470 MR

3.2.1 HEIGHT

The girder height shall be constant throughout the bridge. See chapter 3.1.2 for exceptions when varying span lengths.

The girder height shall be modified to correlate with the viaduct span length.

$$H_{Girder} \leq L_{Span} / 14$$

DBS01-0-2.2-390 PMR

In the diagram as well as the parameter list, it is indicated that a large girder height produces a road cantilever length, which can be below the minimum requirements. It is presented as a note and reminder, since it is possible to adjust other parameters and reach a design, which is within the given design parameters.

3.2.2 UNDERSIDE

The W_{bottom_slab} shall be constant throughout the bridge

$$W_{bottom_slab} = W_{pier_top}$$

The bottom slab shall align with the top width of the pier, although the pier top always shall be horizontal. The bottom can not alter its location with relations to road and rail alignments.
 DBS01-0-4.4-115 MR

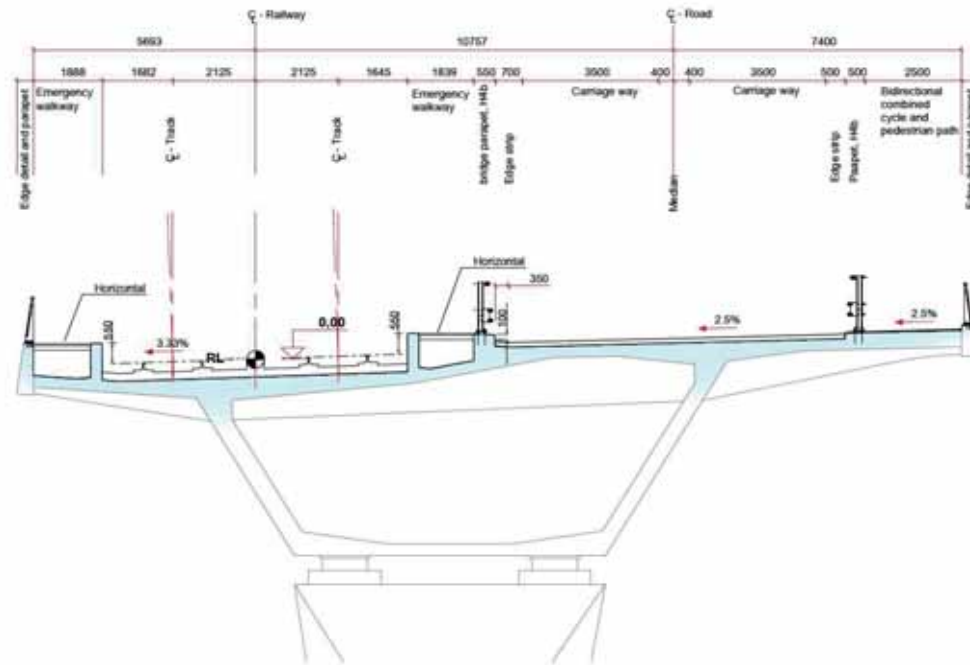
The A_{bottom_slab} shall be constant throughout the bridge.
 DBS01-0-4.4-115 MR

If A_{bottom_slab} is introduced the pier top shall still be horizontal.
 DBS01-0-2.2-475 MR

Two variations can be made to the bottom slab. The width can be altered and a cross slope can be introduced.

GIRDER GEOMETRY AND VARIABLE HEIGHT AND WIDTH + INCLINATIONS / DIAGRAMMATIC SECTIONS. DRAWINGS NOT TO SCALE

NOTES
 See chapter 3.1 for illustrated span normal lengths.



3.2.4 DECK DIMENSIONS

A standard cross section of the top of the girder is shown. All dimensions are locked vertically and horizontally.

The top dimensions of the girder, vertical as well as horizontal, shall be identical to the dimensions that are shown in this section.

DBS01-0-2.4-040 MR
 DBS01-0-2.4-050 PMR
 DBS01-0-2.4-060 MR
 DBS01-0-2.4-070 MR
 DBS01-0-2.5-030 MR
 DBS01-0-2.5-050 MR
 DBS01-0-2.5-070 MR
 DBS01-0-4.4-110 PMR

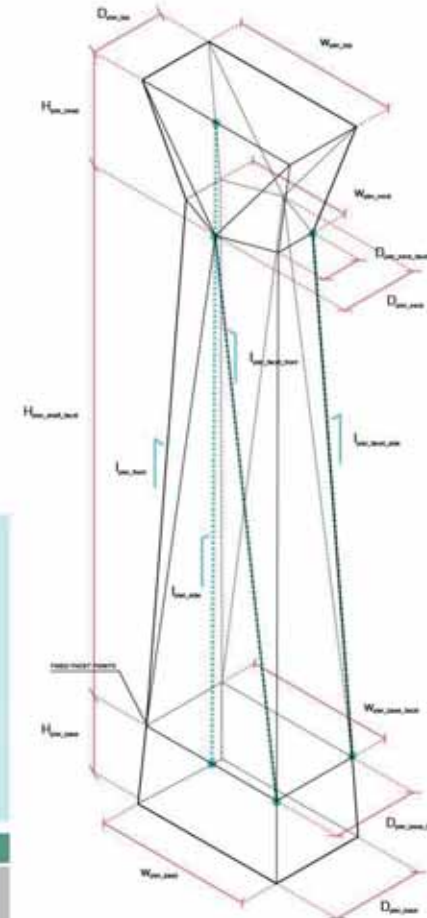
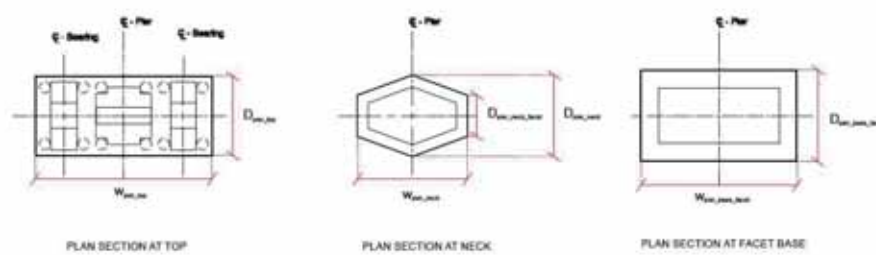
The railway is lowered relative to the road, in order to obtain a horizontal emergency walkway harmonised with a steel parapet and to allow the road users to have a two-sided view of the surroundings.

GIRDER GEOMETRY AND DECK DIMENSIONS / SECTION | 1:100

NOTES

DESIGNED BY: DISSING+WEITLING

DATE: 01.11.2017



3.3.1 GEOMETRY/FACETING

NAME OF PARAMETER	DESCRIPTION	DM*	DESIGN BASIS	PARAMETER VALUES
H_top_pier	Height of pier head	5300	DB01-0.2.2-430 PMR	NT
H_top_neck_pier	Height from pier neck to facet point	24700	-----	NT - only for definition purposes
H_top_base	Height from foundation top to facet point	Varies	-----	N/A - can be <0
W_top_pier	Width of pier top	8000	DB01-1.2.2-410 PMR	W_top_pier = W_facet_base (from: 3502)
W_neck_pier	Width of pier shaft top	5000	DB01-0.2.2-430 PMR	W_neck_pier > 3 (S&T) 1/54E1 aesthetical proportion
W_facet_base_pier	Width at pier base facet	7037	-----	depends on H_top_pier and W_top_pier
W_top_base	Width at pier base	Varies	-----	depends on H_top_pier and W_top_pier and foundation top level
D_top_pier	Depth of pier top	3000	-----	3000 min = (2000+L/40) L = normal viaduct span length in mm
D_top_neck_pier	Depth of pier facet	1860	-----	between 40% and 60% of D_top_pier
D_top_neck_base	Water depth of pier neck	3090	-----	depends on inclination I_top_pier from pier top (side elevation) and D_top_pier
D_top_facet_base_pier	Depth at pier facet base	4105	-----	depends on inclination I_top_pier from pier top (side elevation) and D_top_pier
D_top_base	Depth at pier base	Varies	-----	depends on I_top_pier and D_top_pier and foundation top level
I_top_side	Inclination of pier side	1:120	-----	1:50 to vertical
I_top_facet_side	Inclination of pier facet side	1:22	-----	depends on D_top_pier, H_top_pier, I_top_pier
I_top_facet_front	Inclination of pier facet front	1:7	-----	I_top_facet_base = I_top_pier
I_top_base	Inclination of pier front	1:24	-----	depends on inclination I_top_pier and width W_top_pier

GEOMETRY PARAMETERS / AXONOMETRIC VIEW, PLAN / DRAWINGS NOT TO SCALE

*DIMENSION IN ILLUSTRATIVE DESIGN

DOCUMENT PROVIDED BY: DISSING + WEITLING

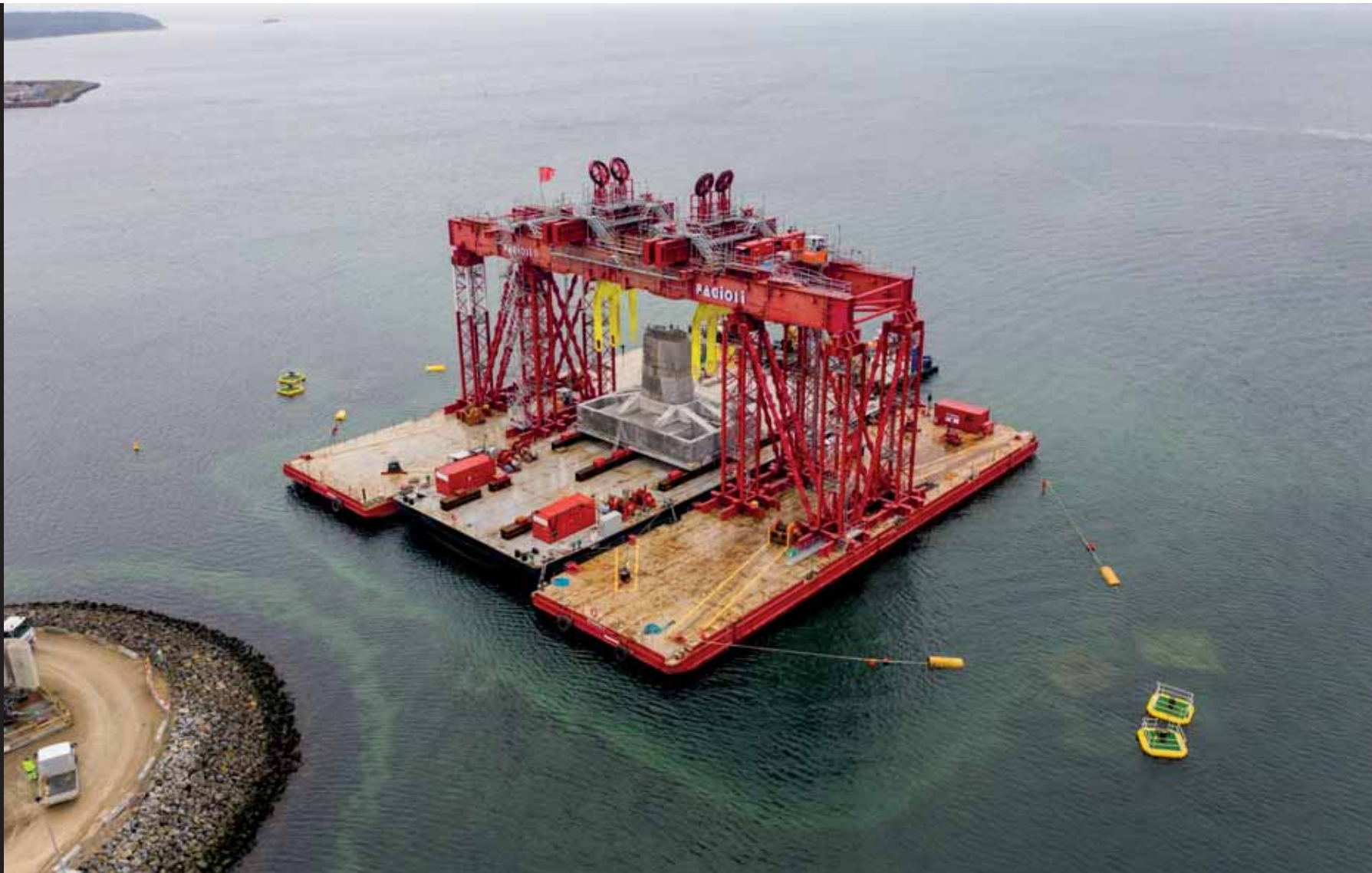
3.3.1 - ALL DEFINITION DESIGN DRAWINGS, BRIDGE / STORSTRØMSBROEN

Foundations 2020:

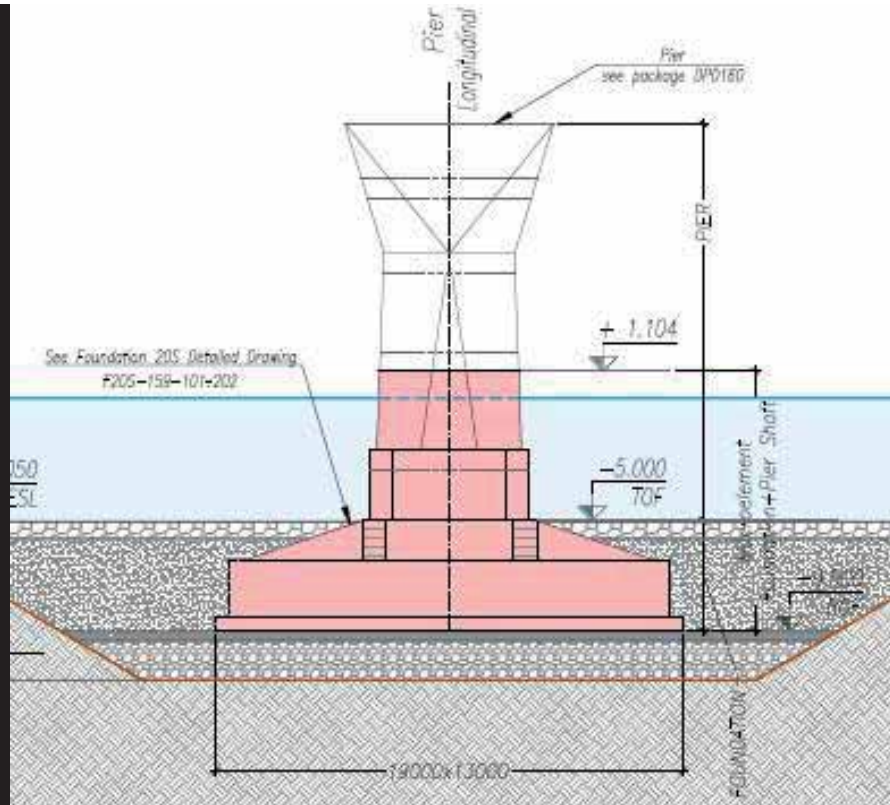
**Weight between 1800-
2500 tons.**

Moved to site on barges



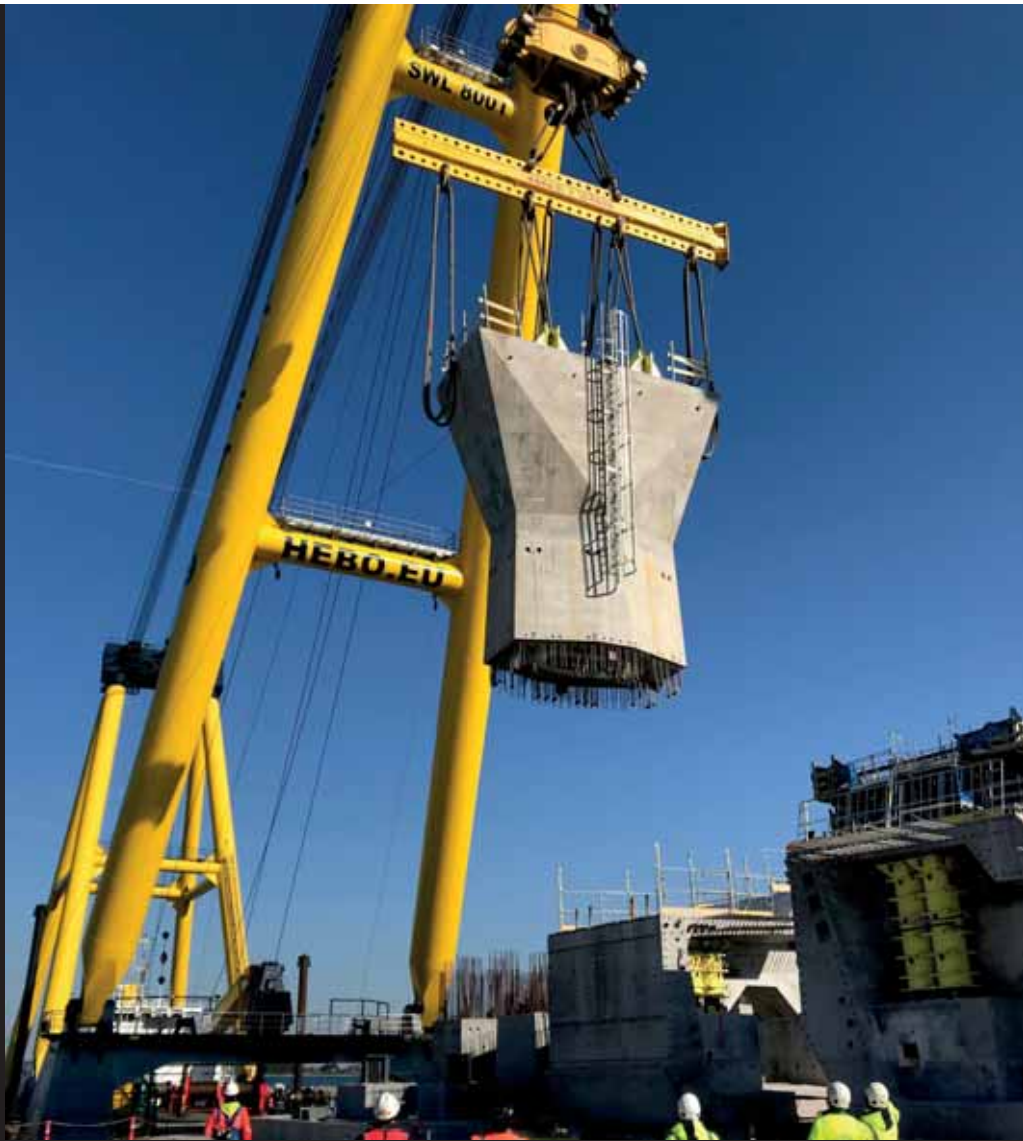


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Prefab bridge deck
Length approx 70 m





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Pylon Foundation moved to barge



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Conclusion:

Denmarks 3'rd longest bridge

A replacement of the existing Storstrøms Bridge

Dissing+Weitling involved in all Phases from Sketching to on-site supervision. Major challenges:

Design and build with definition design

Architectural especially the production of piers was challenging But

Bridge moving forward, delayed, but the architectural intent is kept.

Expected opening for traffic late 2025 and trains in 2028

Dissing + Weitling



"THE CATHEDRALS OF OUR TIME"

Dissing + Weitling